Davidson Drive Holdings LLC c/o ez develop, inc. Israel Vanchozker
Joseph Herskovitz
412 N. Main St
Monroe, NY

## RE: Traffic Impact Study for Proposed Light Industrial Development, Lake Station Road, Town of Chester, Orange County, New York; CM Project No. 121-204

Dear Sirs:
As requested, Creighton Manning Engineering, LLP (CM) has completed a Traffic Impact Study for the proposed industrial development located on Lake Station Road in the Town of Chester, Orange County, NY. This study is based on traffic engineering industry standards and the Preliminary Concept Plan prepared by Arden Consulting Engineering, PLLC, last revised September 16, 2021 which is included under Attachment A.

### 1.0 Project Description

The subject site is defined on the Orange County Tax Map as Section 17, Block 1, Lots 22.1 through 22.8, and is currently undeveloped. The proposed project will construct a new 166,024-square-foot light industrial use building which includes a 4,000 -square-foot office space and be supported by 76 parking spaces inclusive of four ADA-accessible spaces for passenger vehicles and 13 parking spaces for tractor trailers. Vehicular access is proposed via Davidson Drive, currently an unbuilt public paper street which will be terminated by a break-away gate north of the subject site's access driveways that will allow for emergency access. The site will provide two full movement driveways; one of the driveways will be fully dedicated for tractor trailers accessing the site. It is anticipated that the largest shift will consist of 45 employees. The proposed project is expected to be completed and operational by 2023. A map illustrating the site location in relation to the Davidson Drive right-of-way is shown in Exhibit 1.


Exhibit 1 - Site Location

### 2.0 Existing Conditions

Roadways Serving the Site

- Lake Station Road is classified as an Urban Local road and is under the jurisdiction of Town of Chester Highway Department. The roadway runs primarily east-west from Bellvale Road (CR 82) in the Town of Chester to Kings Highway (CR 13) in the Town of Warwick. In the vicinity of the project, Lake Station Road provides one 12-foot-wide travel lane in each direction. Turn lanes are not provided at intersections or driveways. The posted speed limit is 30 miles per hour. There are no sidewalks provided along the roadway.
- Bellvale Road (County Road 82) is classified as an Urban Major Collector roadway and is under the jurisdiction of the Orange County Department of Public Works (OCDPW). The roadway runs primarily north-south from County Road 13 to Gibson Hill Road. In the vicinity of the project, Bellvale Road provides one 12 -foot-wide travel lane in each direction with four-foot-wide shoulders. Turns lanes are not provided at intersections or driveways. The posted speed limit is 45 miles per hour. There are no sidewalks provided along the roadway.
- Kings Highway (County Road 13) is classified as an Urban Minor Collector road and is under the jurisdiction of the OCDPW. The roadway runs primarily north-south from NYS Route 17M to the Village of Warwick. In the vicinity of the project, Kings Highway provides one 11 -foot wide travel lane in each direction with variable width shoulders. Turn lanes are typically not provided at intersections or driveways. The posted speed limit is 55 miles per hour. There are no sidewalks provided along the roadway.
- Paradise Lane: is classified as Urban Local roadway and is under the jurisdiction of Town of Chester Highway Department. The roadway runs north-south from Lake Station Road to the dead end and serves residential homes. The roadway is approximately 38 feet wide. There is no posted speed limit and there are no sidewalks provided along the roadway.


## Study Intersections

- Lake Station Road/Paradise Lane: This is a three-leg unsignalized intersection. The eastbound Lake Station Road approach is uncontrolled and provides one shared lane for through/right-turn movements. The westbound Lake Station Road approach is uncontrolled and provides one shared lane for left-turn/through movements. The northbound Paradise Lane is stop-controlled and provides one shared lane for left-turn/right-turn movements onto Lake Station Road. Exhibit 2 is a Nearmap image that shows the study intersection.


Exhibit 2 - Lake Station Road/Paradise Lane

- Lake Station Road/Bellvale Road: This is a three-leg unsignalized intersection. The eastbound Lake Station Road approach is stop-controlled and provides one shared lane for left-turn/right-turn movements onto Bellvale Road. The northbound Bellvale Road approach is uncontrolled and provides one shared lane for leftturns/through movements. The southbound Bevalle Road provides one shared lane for through/right-turn movements. Exhibit 3 is a Nearmap image that shows the study intersection.
- Lake Station Road/Kings Highway: This is a three-leg unsignalized intersection. The westbound Lake Station Road approach is stopped-controlled and provides one shared lane for left-turn/right-turn movements onto Kings Highway. The northbound Kings Highway approach is uncontrolled and provides one shared lane for through/right-turn movements. The southbound Kings Highway approach is uncontrolled and provides one shared lane for left-turn/through movements. Railroad crossing is located xx -feet east of the intersection. Exhibit 4 is a Nearmap image that shows the study intersection.


## Data Collection



Exhibit 3 - Lake Station Road/Bellvale Road


Turning Movement Counts (TMCs) were conducted at the intersections of Lake Station Road/Kings Highway and Lake Station Road/Bellville Road on Thursday July 22, 2021 from 7:00 AM to 9:00 AM and from 4:00 PM to 6:00 PM. These periods coincide with the anticipated peak-hour operation times of the proposed use as well as the adjacent street traffic. The observed peak hours were 8:00 AM to 9:00 AM and 4:15 PM to 5:15 PM. During 15minute spot counts conducted at the intersection of Lake Station Road/Paradise Lane there were no vehicles observed entering or exiting Paradise Lane. Therefore, CM used the Institute of Transportation Engineers' (ITE) Trip Generation Manual, $10^{\text {th }}$ Edition, to estimate trips entering and exiting based on the seven residential dwelling located along Paradise Lane and those trips being evenly distributed at the intersection of Lake Station Road/Paradise Lane. ${ }^{1}$

It is important to note that the Novel Coronavirus/COVID-19 pandemic was anticipated to have an effect on the turning movement counts. CM cited historical traffic data published by the NYSDOT on the Traffic Data Viewer to compare the observed counts on Kings Highway/Lake Station Road intersection. The comparison showed that the observed AM volumes were lower than historical data, and observed PM volumes were higher than historical data. A calibration factor was applied to the AM volumes to develop pre-pandemic traffic volumes. Figure 1-1 shows the 2021 Existing traffic volumes for the study area. ${ }^{2}$ The raw TMC data is included under Attachment B.

[^0]
### 3.0 Traffic Assessment

## Trip Generation

Trip generation determines the quantity of traffic expected to travel to/from a given site. The Institute of Transportation Engineers' (ITE) Trip Generation Manual, $10^{\text {th }}$ Edition, is the industry-standard resource used for estimating trip generation for proposed land uses based on data collected at similar uses. Upon review of the Trip Generation Manual, Land Use Code (LUC) 110 "General Light Industrial" was applied for the proposed development. It should be noted that the ITE description for LUC 110 states that the study sites are typically inclusive of minimal office space. Table 1 summarizes the trip generation estimate for the weekday AM peak hour and weekday PM peak hour for passenger vehicles (PV) and trucks. ${ }^{3}$

Table 1 - Trip Generation Summary for Proposed Use

| Land Use | Independent Variable | Weekday AM Peak Hour |  |  | Weekday PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Enter | Exit | Total | Enter | Exit | Total |
| General Light Industrial - LUC 110 PV | 166,024 SF | 56 | 7 | 63 | 5 | 45 | 50 |
| General Light Industrial - LUC 110 Trucks | 166,024 SF | 1 | 1 | 2 | 1 | 1 | 2 |
| Total Site-Generated Trips |  | 57 | 8 | 65 | 6 | 46 | 52 |

Table 1 shows that the project is expected to generate 65 total trips during weekday AM peak hour and 52 trips during the weekday PM peak hour. It is important to note that there is no "pass-by" component of the traffic associated with the proposed development. Additionally, the magnitude of the new traffic associated with this development is less than the NYSDOT and ITE threshold of 100-site generated trips on any one intersection.

## Future Traffic Volumes

To evaluate the impact of the proposed project, traffic projections were prepared for the anticipated year of completion - 2023. In order to conservatively forecast the 2023 traffic volume, a $1.0 \%$ growth rate was applied to the existing traffic volumes and compounded annually for two years. CM contacted the Town of Chester Planning Board Chair, Don Serotta, and the Town of Warwick Planning Board Secretary; both Towns identified no other planned development projects that could potentially increase traffic within the study area. The 2023 projected No-Build traffic volumes are shown on Figure 1-2 and represent the traffic volumes without the proposed project.

Traffic generated by the project was distributed on the adjacent roadway based on existing observed travel patterns in the project area and the probable travel routes of truck drivers and employees. The proximity of the site to NYS Route 17 is expected to influence trip-making behavior of the truck drivers. The analysis assumes that all truck trips ( $100 \%$ ) will be drawn to and from Exits 127 and 126 on NYS Route 17. Therefore, truck drivers will utilize the Bellvale Road and Lake Station Road intersection to gain access to and from the site. The distribution of employee vehicles is expected to be more balanced between Kings Highway and Bellvalle Road. In general, all site-generated traffic is expect to ingress and egress the site via the new Davidson Drive connection to Lake Station Road.

The primary trip distribution pattern for the proposed development is shown on Figure 2 for passenger vehicles and Figure 4 for trucks. The associated site-generated traffic volumes are shown on Figures 3 for passenger vehicles and Figure 5 for trucks. The site-generated trips were then added to the 2023 No-Build traffic volumes, resulting in the 2023 Build traffic volumes shown on Figure 6.

[^1]
## Traffic Operations

Intersection Level of Service (LOS) and capacity analysis relate traffic volumes to the physical characteristics of an intersection. Intersection evaluations were made using Synchro Version 11 software, which automates the procedures contained in the Highway Capacity Manual. Table 2 summarizes the results of the level of service calculations for the Existing, No-Build, and Build conditions during the weekday AM peak hour and weekday PM peak hour. The detailed level of service analyses are included under Attachment C .

Table 2 - Level of Service Summary

| Intersection | $\begin{aligned} & \text { oㅁ } \\ & \text { む } \end{aligned}$ | Weekday AM Peak Hour |  |  | Weekday PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{gathered} 2021 \\ \text { Existing } \end{gathered}$ | $2023$ <br> No-Build | $\begin{aligned} & 2023 \\ & \text { Build } \end{aligned}$ | $\begin{gathered} 2021 \\ \text { Existing } \end{gathered}$ | 2023 No-Build | $\begin{aligned} & 2023 \\ & \text { Build } \end{aligned}$ |
| Kings Highway/Lake Station Road | U |  |  |  |  |  |  |
| Lake Station Road, WB LR <br> Kings Highway, SB LT |  | $\begin{gathered} C(16.6) \\ \mathrm{A}(0.7) \end{gathered}$ | $\begin{aligned} & C(16.9) \\ & A(0.7) \\ & \hline \end{aligned}$ | $\begin{aligned} & \mathrm{C}(17.6) \\ & \mathrm{A}(0.8) \\ & \hline \end{aligned}$ | $\begin{gathered} \mathrm{B}(14.6) \\ \mathrm{A}(0.4) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{B}(14.9) \\ \mathrm{A}(0.4) \\ \hline \end{gathered}$ | $\begin{gathered} C(15.5) \\ \mathrm{A}(0.4) \\ \hline \end{gathered}$ |
| Lake Station Road/Paradise Lane/Davidson Drive | U |  |  |  |  |  |  |
| Lake Station Road, EB [L]TR |  | -- | -- | A (1.7) | -- | -- | A (0.2) |
| Lake Station Road, WB LT[R] |  | A (0.1) | A (0.1) | A (0.1) | A (0.2) | A (0.2) | A (0.2) |
| Paradise Lane, NB L[T]R |  | A (8.9) | A (8.9) | A (9.2) | A (8.9) | A (8.9) | A (9.0) |
| [Davidson Drive, SB] [LTR] |  | -- | -- | A (9.9) | -- | -- | A (9.9) |
| Lake Station Road/Bellvale Road | U |  |  |  |  |  |  |
| Lake Station Road, EB LR |  | A (9.3) | A (9.4) | A (9.6) | A (9.4) | A (9.4) | A (9.9) |
| Bellvalle Road, NB LT |  | A (4.0) | A (3.9) | A (4.5) | A (5.2) | A (5.2) | A (5.2) |
|  |  |  |  |  |  |  |  |
| $\mathrm{S}=$ Signalized intersection |  |  |  |  |  |  |  |
| EB, WB, NB, SB = Eastbound, Westbound, Northbound, and Southbound intersection approaches |  |  |  |  |  |  |  |
| L, T, R = Left-turn, Through, and/or Right-turn movements |  |  |  |  |  |  |  |
| $\mathrm{X}(\mathrm{Y} . \mathrm{Y})=$ Level of service (Average delay in seconds per vehicle) |  |  |  |  |  |  |  |

The impact of the project can be described by comparing the analysis of the No-Build and Build operating conditions. The following observation are evident from the analysis:

- Kings Highway/Lake Station Road: The level of service analysis indicates that the minor street approach of the three-leg intersection will operate at an acceptable LOS of C or better in the Build condition, which is consistent with the anticipated LOS for the intersection in the No-Build condition.
- Lake Station Road/Paradise Lane: The level of service indicates that the northbound and southbound minor approaches of the intersection will operate at an acceptable LOS of A or better in the Build condition, which is consistent with the anticipated LOS of the intersection in the No-Build condition.
- Lake Station Road/Bellvale Road: The level of service analysis indicates that the minor street approach of the three-leg intersection will operate at an acceptable LOS of A or better in the Build condition, which is consistent with the anticipated LOS for the intersection in the No-Build condition.


### 4.0 Site Access, Circulation, and Parking

CM reviewed the site access, site circulation and parking layout as shown on the Site Plan prepared by Arden Consulting Engineers, PLLC, last revised September 16, 2021. Vehicular access is proposed via Davidson Drive, currently an unbuilt public paper street. Davidson Drive will provide access to the site via two driveways located approximately 380 and 980 feet north of Lake Station Road and connect to the already built portion of Davidson Drive to the north. However, all site-generated traffic is expected to use Lake Station Road for ingress and egress.

The southerly site access is a 26 -foot-wide driveway dedicated for passenger vehicles only. The northerly site access is a 30 -foot-wide driveway designated for passenger vehicles and trucks; the truck parking area is 152 feet in width, which will allow these vehicles to turn around completely within the confines of the site as necessary. The site will be supported by a total of 76 off-street parking spaces inclusive of four ADA-accessible spaces and 13 parking spaces for trucks. The proposed number of off-street parking for passenger vehicles meets the Town of Chester zoning requirements for the combined total of the office use component ( 1 space/200SF) and the industrial use component (2 spaces/3 employees).

### 5.0 Conclusion

The subject site is located on the parcel defined as Section 17, Block 1, Lots 22.1 through 22.8 on the Orange County Map. The proposed project will construct a new 166,024 -square-foot light industrial use building which includes a 4,000 -square-foot office space. It is anticipated that the largest shift will consist of 45 employees. The following is noted regarding the proposed project:

- Turning movement counts were conducted on Thursday July 22, 2021, from 7:00 AM to 9:00 AM and from 4:00 PM to 6:00 PM. The morning peak hour occurred from 8:00 AM to 9:00 AM, and evening peak hour occurred from 4:15 PM to 5:15 PM.
- Trips generated by the seven residential dwellings located on Paradise Lane were estimated using ITE LUC 210 - "Single-Family Detached Housing" and then evenly distributed onto Lake Station Road.
- The site is expected to generate 65 total trips during the weekday morning peak hour and 52 total trips during the weekday evening peak hour.
- The level of service analysis indicates that the Build condition of the study intersections, Kings Highway/Lake Station Road, Lake Station Road/Davidson Drive/Paradise Lane, and Bellvale Road/Lake Station Road, will operate at the level of service consistent with the No-Build conditions.
- The project is not expected to have a significant adverse impact on surrounding roadway network.

Please do not hesitate to call our office if you have any questions or comments, or require additional information.
Respectfully submitted,
Creighton Manning Engineering, LLP


Frank A. Filiciotto, PE
Associate
cc:


Project Engineer
Project Engineer
(1)

(2)

2023 NO-BUILD TRAFFIC VOLUMES


LEGEND:
AM (PM)




LEGEND:
ENTERING (EXITING)



# ATTACHMENT A SITE PLAN 

PROPOSED LIGHT INDUSTRIAL DEVELOPMENT<br>LAKE STATION ROAD<br>TOWN OF CHESTER<br>ORANGE COUNTY, NEW YORK



# ATTACHMENT B TURNING MOVEMENT COUNTS 

PROPOSED LIGHT INDUSTRIAL DEVELOPMENT LAKE STATION ROAD
TOWN OF CHESTER ORANGE COUNTY, NEW YORK

ID: 857862, Location: 41.29903, -74.277448


* L: Left, R: Right, T: Thru, U: U-Turn
[N] Bellvale Rd
Total: 165
In: 64 Out: 101
$\stackrel{\oplus}{\sim} \stackrel{\infty}{m}$


Out: $120 \quad$ In: 139
Total: 259
[S] Bellvale Rd

AM Peak (8 AM - 9 AM) - Overall Peak Hour
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857862, Location: 41.29903, -74.277448

*L: Left, R: Right, T: Thru, U: U-Turn
[N] Bellvale Rd
Total: 89
In: $31 \quad$ Out: 58
-


Out: $66 \quad$ In: 78
Total: 144
[S] Bellvale Rd

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857863, Location: 41.29903, -74.277448

| Leg Direction |  | Lake Station <br> Eastbound |  |  |  | Bellvale Rd <br> Northbound |  |  |  | Bellvale Rd <br> Southbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time |  | L | R | U | App | L | T | U | App | T | R | U | App | Int |
|  | 2021-07-22 4:00PM | 5 | 14 | 0 | 19 | 16 | 12 | 0 | 28 | 7 | 5 | 0 | 12 | 59 |
|  | 4:15PM | 5 | 26 | 0 | 31 | 20 | 8 | 0 | 28 | 10 | 3 | 0 | 13 | 72 |
|  | 4:30PM | 6 | 15 | 0 | 21 | 22 | 6 | 0 | 28 | 8 | 6 | 0 | 14 | 63 |
|  | 4:45PM | 2 | 13 | 0 | 15 | 13 | 10 | 0 | 23 | 15 | 3 | 0 | 18 | 56 |
|  | Hourly Total | 18 | 68 | 0 | 86 | 71 | 36 | 0 | 107 | 40 | 17 | 0 | 57 | 250 |
|  | 5:00PM | 3 | 12 | 0 | 15 | 15 | 13 | 0 | 28 | 15 | 4 | 0 | 19 | 62 |
|  | 5:15PM | 2 | 11 | 0 | 13 | 16 | 13 | 0 | 29 | 11 | 5 | 0 | 16 | 58 |
|  | 5:30PM | 3 | 16 | 0 | 19 | 16 | 12 | 0 | 28 | 13 | 3 | 0 | 16 | 63 |
|  | 5:45PM | 3 | 9 | 0 | 12 | 16 | 15 | 0 | 31 | 13 | 3 | 0 | 16 | 59 |
|  | Hourly Total | 11 | 48 | 0 | 59 | 63 | 53 | 0 | 116 | 52 | 15 | 0 | 67 | 242 |
|  | 6:00PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Hourly Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Total | 29 | 116 | 0 | 145 | 134 | 89 | 0 | 223 | 92 | 32 | 0 | 124 | 492 |
|  | \% Approach | 20.0\% | 80.0\% | 0\% | - | 60.1\% | 39.9\% | 0\% |  | 74.2\% | 25.8\% | 0\% | - |  |
|  | \% Total | 5.9\% | 23.6\% | 0\% | 29.5\% | 27.2\% | 18.1\% | 0\% | 45.3\% | 18.7\% | 6.5\% | 0\% | 25.2\% |  |
|  | Lights | 29 | 114 | 0 | 143 | 132 | 87 | 0 | 219 | 90 | 32 | 0 | 122 | 484 |
|  | \% Lights | 100\% | 98.3\% | 0\% | 98.6\% | 98.5\% | 97.8\% | 0\% | 98.2\% | 97.8\% | 100\% | 0\% | 98.4\% | 98.4\% |
|  | Articulated Trucks and Single-Unit Trucks | 0 | 1 | 0 | 1 | 1 | 2 | 0 | 3 | 2 | 0 | 0 | 2 | 6 |
|  | \% Articulated Trucks and Single-Unit Trucks | 0\% | 0.9\% | 0\% | 0.7\% | 0.7\% | 2.2\% | 0\% | 1.3\% | 2.2\% | 0\% | 0\% | 1.6\% | 1.2\% |
|  | Buses | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 2 |
|  | \% Buses | 0\% | 0.9\% | 0\% | 0.7\% | 0.7\% | 0\% | 0\% | 0.4\% | 0\% | 0\% | 0\% | 0\% | 0.4\% |

*L: Left, R: Right, T: Thru, U: U-Turn
[N] Bellvale Rd
Total: 242
In: 124 Out: 118
N Ñ


Out: 208 In: 223
Total: 431
[S] Bellvale Rd

PM Peak (4:15 PM - 5:15 PM) - Overall Peak Hour
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857863, Location: 41.29903, -74.277448

| Leg Direction |  | Lake Station Eastbound |  |  |  | Bellvale Rd Northbound |  |  |  | Bellvale Rd Southbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time |  | L | R | U | App | L | T | U | App | T | R | U | App | Int |  |
|  | 2021-07-22 4:15PM | 5 | 26 | 0 | 31 | 20 | 8 | 0 | 28 | 10 | 3 | 0 | 13 |  | 72 |
|  | 4:30PM | 6 | 15 | 0 | 21 | 22 | 6 | 0 | 28 | 8 | 6 | 0 | 14 |  | 63 |
|  | 4:45PM | 2 | 13 | 0 | 15 | 13 | 10 | 0 | 23 | 15 | 3 | 0 | 18 |  | 56 |
|  | 5:00PM | 3 | 12 | 0 | 15 | 15 | 13 | 0 | 28 | 15 | 4 | 0 | 19 |  | 62 |
|  | Total | 16 | 66 | 0 | 82 | 70 | 37 | 0 | 107 | 48 | 16 | 0 | 64 |  | 253 |
|  | \% Approach | 19.5\% | 80.5\% | 0\% | - | 65.4\% | 34.6\% | 0\% | - | 75.0\% | 25.0\% | 0\% | - |  |  |
|  | \% Total | 6.3\% | 26.1\% | 0\% | 32.4\% | 27.7\% | 14.6\% | 0\% | 42.3\% | 19.0\% | 6.3\% | 0\% | 25.3\% |  |  |
|  | PHF | 0.667 | 0.635 | - | 0.661 | 0.795 | 0.712 | - | 0.955 | 0.800 | 0.667 | - | 0.842 |  | 0.878 |
|  | Lights | 16 | 64 | 0 | 80 | 69 | 36 | 0 | 105 | 46 | 16 | 0 | 62 |  | 247 |
|  | \% Lights | 100\% | 97.0\% | 0\% | 97.6\% | 98.6\% | 97.3\% | 0\% | 98.1\% | 95.8\% | 100\% | 0\% | 96.9\% |  | 97.6\% |
|  | Articulated Trucks and Single-Unit Trucks | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 2 | 0 | 0 | 2 |  | 4 |
|  | \% Articulated Trucks and Single-Unit Trucks | 0\% | 1.5\% | 0\% | 1.2\% | 0\% | 2.7\% | 0\% | 0.9\% | 4.2\% | 0\% | 0\% | 3.1\% |  | 1.6\% |
|  | Buses | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |  | 2 |
|  | \% Buses | 0\% | 1.5\% | 0\% | 1.2\% | 1.4\% | 0\% | 0\% | 0.9\% | 0\% | 0\% | 0\% | 0\% |  | 0.8\% |

*L: Left, R: Right, T: Thru, U: U-Turn
[N] Bellvale Rd
Total: 117
In: 64 Out: 53
$\stackrel{\infty}{\square}$


Total: 221
[S] Bellvale Rd

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857857, Location: 41.295737, -74.291344

| Leg Direction |  | Lake Station Westbound |  |  |  | Kings Hwy <br> Northbound |  |  |  | Kings Hwy Southbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time |  | L | R | U | App | T | R | U | App | L | T | U | App | Int |
|  | 2021-07-22 7:00AM | 7 | 3 | 0 | 10 | 25 | 3 | 0 | 28 | 2 | 16 | 0 | 18 | 56 |
|  | 7:15AM | 7 | 2 | 0 | 9 | 33 | 8 | 0 | 41 | 3 | 27 | 0 | 30 | 80 |
|  | 7:30AM | 11 | 2 | 0 | 13 | 37 | 9 | 0 | 46 | 3 | 37 | 0 | 40 | 99 |
|  | 7:45AM | 13 | 1 | 0 | 14 | 47 | 9 | 0 | 56 | 3 | 30 | 0 | 33 | 103 |
|  | Hourly Total | 38 | 8 | 0 | 46 | 142 | 29 | 0 | 171 | 11 | 110 | 0 | 121 | 338 |
|  | 8:00AM | 10 | 0 | 0 | 10 | 27 | 12 | 0 | 39 | 4 | 36 | 0 | 40 | 89 |
|  | 8:15AM | 14 | 1 | 0 | 15 | 40 | 15 | 0 | 55 | 3 | 30 | 0 | 33 | 103 |
|  | 8:30AM | 11 | 4 | 0 | 15 | 40 | 13 | 0 | 53 | 3 | 42 | 0 | 45 | 113 |
|  | 8:45AM | 16 | 1 | 0 | 17 | 56 | 7 | 0 | 63 | 5 | 65 | 1 | 71 | 151 |
|  | Hourly Total | 51 | 6 | 0 | 57 | 163 | 47 | 0 | 210 | 15 | 173 | 1 | 189 | 456 |
|  | 9:00AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Hourly Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  | Total | 89 | 14 | 0 | 103 | 305 | 76 | 0 | 381 | 26 | 283 | 1 | 310 | 794 |
|  | \% Approach | 86.4\% | 13.6\% | 0\% |  | 80.1\% | 19.9\% | 0\% | - | 8.4\% | 91.3\% | 0.3\% | - |  |
|  | \% Total | 11.2\% | 1.8\% | 0\% | 13.0\% | 38.4\% | 9.6\% | 0\% | 48.0\% | 3.3\% | 35.6\% | 0.1\% | 39.0\% |  |
|  | Lights | 85 | 13 | 0 | 98 | 283 | 71 | 0 | 354 | 24 | 266 | 1 | 291 | 743 |
|  | \% Lights | 95.5\% | 92.9\% | 0\% | 95.1\% | 92.8\% | 93.4\% | 0\% | 92.9\% | 92.3\% | 94.0\% | 100\% | 93.9\% | 93.6\% |
|  | Articulated Trucks and Single-Unit Trucks | 3 | 0 | 0 | 3 | 17 | 3 | 0 | 20 | 1 | 13 | 0 | 14 | 37 |
|  | \% Articulated Trucks and Single-Unit Trucks | 3.4\% | 0\% | 0\% | 2.9\% | 5.6\% | 3.9\% | 0\% | 5.2\% | 3.8\% | 4.6\% | 0\% | 4.5\% | 4.7\% |
|  | Buses | 1 | 1 | 0 | 2 | 5 | 2 | 0 | 7 | 1 | 4 | 0 | 5 | 14 |
|  | \% Buses | 1.1\% | 7.1\% | 0\% | 1.9\% | 1.6\% | 2.6\% | 0\% | 1.8\% | 3.8\% | 1.4\% | 0\% | 1.6\% | 1.8\% |

*L: Left, R: Right, T: Thru, U: U-Turn

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857857, Location: 41.295737, -74.291344

Provided by: Creighton Manning Engineering, LLP 2 Winners Circle, Albany, NY, 12205, US

## [N] Kings Hwy

Total: 630
In: 310 Out: 320
$\stackrel{\sim}{\sim} \stackrel{\bullet}{\sim}-$


In: 381
Total: 753
[S] Kings Hwy

AM Peak (8 AM - 9 AM) - Overall Peak Hour
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857857, Location: 41.295737, -74.291344

| $\begin{array}{\|l\|} \hline \text { Leg } \\ \text { Direction } \end{array}$ |  | Lake Station Westbound |  |  |  | Kings Hwy <br> Northbound |  |  |  | Kings Hwy <br> Southbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time |  | L | R | U | App | T | R | U | App | L | T | U | App | Int |
|  | 2021-07-22 8:00AM | 10 | 0 | 0 | 10 | 27 | 12 | 0 | 39 | 4 | 36 | 0 | 40 | 89 |
|  | 8:15AM | 14 | 1 | 0 | 15 | 40 | 15 | 0 | 55 | 3 | 30 | 0 | 33 | 103 |
|  | 8:30AM | 11 | 4 | 0 | 15 | 40 | 13 | 0 | 53 | 3 | 42 | 0 | 45 | 113 |
|  | 8:45AM | 16 | 1 | 0 | 17 | 56 | 7 | 0 | 63 | 5 | 65 | 1 | 71 | 151 |
|  | Total | 51 | 6 | 0 | 57 | 163 | 47 | 0 | 210 | 15 | 173 | 1 | 189 | 456 |
|  | \% Approach | 89.5\% | 10.5\% | 0\% |  | 77.6\% | 22.4\% | 0\% |  | 7.9\% | 91.5\% | 0.5\% |  |  |
|  | \% Total | 11.2\% | 1.3\% | 0\% | 12.5\% | 35.7\% | 10.3\% | 0\% | 46.1\% | 3.3\% | 37.9\% | 0.2\% | 41.4\% |  |
|  | PHF | 0.797 | 0.375 | - | 0.838 | 0.728 | 0.783 | - | 0.833 | 0.750 | 0.665 | 0.250 | 0.665 | 0.755 |
|  | Lights | 48 | 6 | 0 | 54 | 153 | 44 | 0 | 197 | 14 | 162 | 1 | 177 | 428 |
|  | \% Lights | 94.1\% | 100\% | 0\% | 94.7\% | 93.9\% | 93.6\% | 0\% | 93.8\% | 93.3\% | 93.6\% | 100\% | 93.7\% | 93.9\% |
|  | Articulated Trucks and Single-Unit Trucks | 2 | 0 | 0 | 2 | 9 | 2 | 0 | 11 | 1 | 8 | 0 | 9 | 22 |
|  | \% Articulated Trucks and Single-Unit Trucks | 3.9\% | 0\% | 0\% | 3.5\% | 5.5\% | 4.3\% | 0\% | 5.2\% | 6.7\% | 4.6\% | 0\% | 4.8\% | 4.8\% |
|  | Buses | 1 | 0 | 0 | 1 | 1 | 1 | 0 | 2 | 0 | 3 | 0 | 3 | 6 |
|  | \% Buses | 2.0\% | 0\% | 0\% | 1.8\% | 0.6\% | 2.1\% | 0\% | 1.0\% | 0\% | 1.7\% | 0\% | 1.6\% | 1.3\% |

*L: Left, R: Right, T: Thru, U: U-Turn

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857857, Location: 41.295737, -74.291344

Provided by: Creighton Manning Engineering, LLP

## [N] Kings Hwy

Total: 359
In: $189 \quad$ Out: 170
$\stackrel{n}{\underset{\sim}{n}} \quad \stackrel{\text { n }}{\sim}$

$\stackrel{v}{\bullet}$
Out: $62 \quad$ In: 57
Total: 119
[E] Lake Station Rd

Out: 224 In: 210
Total: 434
[S] Kings Hwy

Full Length (4 PM-6 PM)
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857858, Location: 41.295737, -74.291344

Provided by: Creighton Manning Engineering, LLP 2 Winners Circle, Albany, NY, 12205, US

*L: Left, R: Right, T: Thru, U: U-Turn

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857858, Location: 41.295737, -74.291344

Provided by: Creighton Manning Engineering, LLP 2 Winners Circle, Albany, NY, 12205, US

## [N] Kings Hwy

Total: 982
In: $489 \quad$ Out: 493
$\stackrel{\infty}{\circ}$ 안


[^2]Out: 606 In: 580 Total: 1186
[S] Kings Hwy

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857858, Location: 41.295737, -74.291344

| Leg Direction |  | Lake Station Westbound |  |  |  | Kings Hwy <br> Northbound |  |  |  | Kings Hwy Southbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time |  | L | R | U | App | T | R | U | App | L | T | U | App | Int |  |
|  | 2021-07-22 4:30PM | 22 | 11 | 0 | 33 | 47 | 19 | 0 | 66 | 5 | 53 | 0 | 58 |  | 157 |
|  | 4:45PM | 12 | 4 | 0 | 16 | 56 | 14 | 0 | 70 | 4 | 70 | 0 | 74 |  | 160 |
|  | 5:00PM | 22 | 6 | 0 | 28 | 54 | 13 | 0 | 67 | 4 | 61 | 0 | 65 |  | 160 |
|  | 5:15PM | 12 | 3 | 0 | 15 | 71 | 19 | 0 | 90 | 0 | 62 | 1 | 63 |  | 168 |
|  | Total | 68 | 24 | 0 | 92 | 228 | 65 | 0 | 293 | 13 | 246 | 1 | 260 |  | 645 |
|  | \% Approach | 73.9\% | 26.1\% | 0\% |  | 77.8\% | 22.2\% | 0\% |  | 5.0\% | 94.6\% | 0.4\% |  |  |  |
|  | \% Total | 10.5\% | 3.7\% | 0\% | 14.3\% | 35.3\% | 10.1\% | 0\% | 45.4\% | 2.0\% | 38.1\% | 0.2\% | 40.3\% |  |  |
|  | PHF | 0.773 | 0.545 | - | 0.697 | 0.803 | 0.855 | - | 0.814 | 0.650 | 0.879 | 0.250 | 0.878 |  | 0.960 |
|  | Lights | 68 | 23 | 0 | 91 | 224 | 64 | 0 | 288 | 13 | 242 | 1 | 256 |  | 635 |
|  | \% Lights | 100\% | 95.8\% | 0\% | 98.9\% | 98.2\% | 98.5\% | 0\% | 98.3\% | 100\% | 98.4\% | 100\% | 98.5\% |  | 98.4\% |
|  | Articulated Trucks and Single-Unit Trucks | 0 | 1 | 0 | 1 | 3 | 0 | 0 | 3 | 0 | 3 | 0 | 3 |  | 7 |
|  | \% Articulated Trucks and Single-Unit Trucks | 0\% | 4.2\% | 0\% | 1.1\% | 1.3\% | 0\% | 0\% | 1.0\% | 0\% | 1.2\% | 0\% | 1.2\% |  | 1.1\% |
|  | Buses | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 2 | 0 | 1 | 0 | 1 |  | 3 |
|  | \% Buses | 0\% | 0\% | 0\% | 0\% | 0.4\% | 1.5\% | 0\% | 0.7\% | 0\% | 0.4\% | 0\% | 0.4\% |  | 0.5\% |

* L: Left, R: Right, T: Thru, U: U-Turn

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)
All Movements
ID: 857858, Location: 41.295737, -74.291344

Creighton

## [N] Kings Hwy

Total: 513
In: $260 \quad$ Out: 253
$\stackrel{0}{\underset{\sim}{\sim}} \stackrel{m}{\square}$

$\stackrel{\circ}{\sim}$
Out: 78 In: 92
Total: 170
[E] Lake Station Rd

Out: 314 In: 293
Total: 607
[S] Kings Hwy

# ATTACHMENT C LEVEL OF SERVICE ANALYSIS 

PROPOSED LIGHT INDUSTRIAL DEVELOPMENT<br>LAKE STATION ROAD<br>TOWN OF CHESTER<br>ORANGE COUNTY, NEW YORK

## LOS Definitions

The following is an excerpt from the Highway Capacity Manual, $6^{\text {th }}$ Edition (HCM).

## Level of Service for Signalized Intersections

Level of Service (LOS) can be characterized for the entire intersection, each intersection approach, and each lane group. Control delay alone is used to characterize LOS for the entire intersection or an approach. Control delay and volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio are used to characterize LOS for a lane group. Delay quantifies the increase in travel time due to traffic signal control. It is also a surrogate measure of driver discomfort and fuel consumption. The v/c ratio quantifies the degree to which a phase's capacity is utilized by a lane group. The following paragraphs describe each LOS.

LOS A describes operations with a control delay of $10 \mathrm{~s} / \mathrm{veh}$ or less and a $\mathrm{v} / \mathrm{c}$ ratio no greater than 1.0. This level is typically assigned when the $\mathrm{v} / \mathrm{c}$ ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

LOS B describes operations with control delay between 10 and $20 \mathrm{~s} / \mathrm{veh}$ and a v/c ratio no greater than 1.0. This level is typically assigned when the $\mathrm{v} / \mathrm{c}$ ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

LOS C describes operations with control delay between 20 and $35 \mathrm{~s} / \mathrm{veh}$ and a $\mathrm{v} / \mathrm{c}$ ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

LOS D describes operations with control delay between 35 and $55 \mathrm{~s} / \mathrm{veh}$ and a v/c ratio no greater than 1.0. This level is typically assigned when the $\mathrm{v} / \mathrm{c}$ ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

LOS E describes operations with control delay between 55 and $80 \mathrm{~s} / \mathrm{veh}$ and a v/c ratio no greater than 1.0. This level is typically assigned when the $\mathrm{v} / \mathrm{c}$ ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F describes operations with control delay exceeding $80 \mathrm{~s} /$ veh or a v/c ratio greater than 1.0. This level is typically assigned when the $v / c$ ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

A lane group can incur a delay less than $80 \mathrm{~s} / \mathrm{veh}$ when the $\mathrm{v} / \mathrm{c}$ ratio exceeds 1.0 . This condition typically occurs when the cycle length is short, the signal progression is favorable, or both. As a result, both the delay and v/c ratio are considered when lane group LOS is established. A ratio of 1.0 or more indicates that cycle capacity is fully utilized and represents failure from a capacity perspective (just as delay in excess of $80 \mathrm{~s} /$ veh represents failure from a delay perspective).

Average control delay and queue length at roundabout controlled intersections are calculated using SIDRA Intersection. The physical geometry such as entry lane width and approach flare, and traffic volume at the roundabout are factors that influence the intersection's performance. The average delay reported using SIDRA Intersection is based on the signalized HCM Method of Delay for Level-of-Service.

## Level of Service Criteria for Unsignalized Intersections

Level of service (LOS) for Two-Way Stop-Controlled (TWSC) intersections is determined by the computed or measured control delay. For motor vehicles, LOS is determined for each minor-street movement (or shared movement) as well as major-street left turns by using criteria given in Exhibit 20-2. LOS is not defined for the intersection as a whole or for major-street approaches for three primary reasons: (a) major-street through vehicles are assumed to experience zero delay; (b) the disproportionate number of major-street through vehicles at a typical TWSC intersection skews the weighted average of all movements, resulting in a very low overall average delay for all vehicles; and (c) the resulting low delay can mask important LOS deficiencies for minor movements. LOS F is assigned to the movement if the volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio for the movement exceeds 1.0, regardless of the control delay.

The LOS criteria for TWSC intersections are somewhat different from the criteria used in Chapter 18 for signalized intersections, primarily because user perceptions differ among transportation facility types. The expectation is that a signalized intersection is designed to carry higher traffic volumes and will present greater delay than an unsignalized intersection. Unsignalized intersections are also associated with more uncertainty for users, as delays are less predictable than they are at signals, which can reduce users' delay tolerance.

The LOS criteria for All-Way Stop-Controlled (AWSC) intersections are given in Exhibit 21-8. LOS $F$ is assigned if the v/c ratio of a lane exceeds 1.0, regardless of the control delay. For assessment of LOS at the approach and intersection levels, LOS is based solely on control delay.

Exhibits 20-2/21-8:
Level-of-Service Criteria for Stop Controlled Intersections

| Control Delay (s/veh) | LOS by Volume-to-Capacity Ratio |  |
| :---: | :---: | :---: |
|  | $\mathbf{v} / \mathrm{c} \leq \mathbf{1 . 0}$ | $\mathbf{v} / \mathrm{c} \geq \mathbf{1 . 0}$ |
| 10.0 | A | F |
| $>10.0$ and $\leq 15.0$ | B | F |
| $>15.0$ and $\leq 25.0$ | C | F |
| $>25.0$ and $\leq 35.0$ | D | F |
| $>35.0$ and $\leq 50.0$ | E | F |
| $>50.0$ | F | F |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 56 | 7 | 179 | 56 | 18 | 190 |
| Future Vol, veh/h | 56 | 7 | 179 | 56 | 18 | 190 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 7 | - | 4 | - | - | 3 |
| Peak Hour Factor | 76 | 76 | 76 | 76 | 76 | 76 |
| Heavy Vehicles, \% | 6 | 0 | 6 | 6 | 7 | 6 |
| Mvmt Flow | 74 | 9 | 236 | 74 | 24 | 250 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 571 | 273 | 0 | 0 | 310 | 0 |
| Stage 1 | 273 | - | - | - | - | - |
| Stage 2 | 298 | - | - | - | - | - |
| Critical Hdwy | 7.86 | 6.9 | - | - | 4.17 | - |
| Critical Hdwy Stg 1 | 6.86 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 6.86 | - | - | - | - | - |
| Follow-up Hdwy | 3.554 | 3.3 | - | - | 2.263 | - |
| Pot Cap-1 Maneuver | 381 | 731 | - | - | 1223 | - |
| Stage 1 | 687 | - | - | - | - | - |
| Stage 2 | 663 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 372 | 731 | - | - | 1223 | - |
| Mov Cap-2 Maneuver | 372 | - | - | - | - | - |
| Stage 1 | 687 | - | - | - | - | - |
| Stage 2 | 648 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 16.6 |  | 0 |  | 0.7 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRV | VBLn1 | SBL |  |
| Capacity (veh/h) |  | - | - | 393 | 1223 | - |
| HCM Lane V/C Ratio |  | - | - | 0.211 | 0.019 | - |
| HCM Control Delay (s) |  | - | - | 16.6 | 8 | 0 |
| HCM Lane LOS |  | - | - | C | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.8 | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | -1 | ric |  |
| Traffic Vol, veh/h | 73 | 1 | 1 | 59 | 4 | 4 |
| Future Vol, veh/h | 73 | 1 | 1 | 59 | 4 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 1 | - | - | -5 | -7 | - |
| Peak Hour Factor | 80 | 80 | 80 | 80 | 80 | 80 |
| Heavy Vehicles, \% | 5 | 0 | 0 | 4 | 0 | 0 |
| Mvmt Flow | 91 | 1 | 1 | 74 | 5 | 5 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.3 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | -1 | F |  |
| Traffic Vol, veh/h | 24 | 53 | 47 | 41 | 22 | 13 |
| Future Vol, veh/h | 24 | 53 | 47 | 41 | 22 | 13 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | -2 | - | - | 0 | 0 | - |
| Peak Hour Factor | 80 | 80 | 80 | 80 | 80 | 80 |
| Heavy Vehicles, \% | 10 | 2 | 5 | 5 | 15 | 0 |
| Mvmt Flow | 30 | 66 | 59 | 51 | 28 | 16 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M |  | T |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 73 | 26 | 213 | 71 | 14 | 239 |
| Future Vol, veh/h | 73 | 26 | 213 | 71 | 14 | 239 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, | 0 | - | 0 | - | - | 0 |
| Grade, \% | 7 | - | 4 | - | - | 3 |
| Peak Hour Factor | 99 | 99 | 99 | 99 | 99 | 99 |
| Heavy Vehicles, \% | 0 | 4 | 2 | 2 | 0 | 2 |
| Mvmt Flow | 74 | 26 | 215 | 72 | 14 | 241 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 520 | 251 | 0 | 0 | 287 | 0 |
| Stage 1 | 251 | - | - | - | - | - |
| Stage 2 | 269 | - | - | - | - | - |
| Critical Hdwy | 7.8 | 6.94 | - | - | 4.1 | - |
| Critical Hdwy Stg 1 | 6.8 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 6.8 | - | - | - | - | - |
| Follow-up Hdwy | 3.5 | 3.336 | - | - | 2.2 | - |
| Pot Cap-1 Maneuver | 425 | 746 | - | - | 1287 | - |
| Stage 1 | 722 | - | - | - | - | - |
| Stage 2 | 703 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 419 | 746 | - | - | 1287 | - |
| Mov Cap-2 Maneuver | 419 | - | - | - | - | - |
| Stage 1 | 722 | - | - | - | - | - |
| Stage 2 | 694 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 14.6 |  | 0 |  | 0.4 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - |  | 474 | 1287 | - |
| HCM Lane V/C Ratio |  | - | - | 0.211 | 0.011 | - |
| HCM Control Delay (s) |  | - | - | 14.6 | 7.8 | 0 |
| HCM Lane LOS |  | - | - | B | A | A |
| HCM 95th \%tile Q(veh) |  | - |  | 0.8 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | -1 | Tr |  |
| Traffic Vol, veh/h | 83 | 2 | 3 | 98 | 1 | 2 |
| Future Vol, veh/h | 83 | 2 | 3 | 98 | 1 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 1 | - | - | -5 | -7 | - |
| Peak Hour Factor | 88 | 88 | 88 | 88 | 88 | 88 |
| Heavy Vehicles, \% | 2 | 0 | 0 | 1 | 0 | 0 |
| Mvmt Flow | 94 | 2 | 3 | 111 | 1 | 2 |


| Major/Minor | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 96 | 0 | 212 | 95 |
| Stage 1 | - | - | - | - | 95 | - |
| Stage 2 | - | - | - | - | 117 | - |
| Critical Hdwy | - | - | 4.1 | - | 5 | 5.5 |
| Critical Hdwy Stg 1 | - | - | - | - | 4 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 4 | - |
| Follow-up Hdwy | - | - | 2.2 | - | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | - | - | 1510 | - | 848 | 985 |
| Stage 1 | - | - | - | - | 969 | - |
| Stage 2 | - | - | - | - | 956 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1510 | - | 846 | 985 |
| Mov Cap-2 Maneuver | - | - | - | - | 846 | - |
| Stage 1 | - | - | - | - | 969 | - |
| Stage 2 | - | - | - | - | 954 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0.2 |  | 8.9 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) |  | 934 | - | - | 1510 | - |
| HCM Lane V/C Ratio |  | 0.004 | - |  | 0.002 | - |
| HCM Control Delay (s) |  | 8.9 | - | - | 7.4 | 0 |
| HCM Lane LOS |  | A | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.2 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | r |  |  | - | 个 |  |
| Traffic Vol, veh/h | 16 | 69 | 82 | 37 | 48 | 19 |
| Future Vol, veh/h | 16 | 69 | 82 | 37 | 48 | 19 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | -2 | - | - | 0 | 0 | - |
| Peak Hour Factor | 88 | 88 | 88 | 88 | 88 | 88 |
| Heavy Vehicles, \% | 0 | 3 | 1 | 3 | 4 | 0 |
| Mvmt Flow | 18 | 78 | 93 | 42 | 55 | 22 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 57 | 7 | 183 | 57 | 18 | 194 |
| Future Vol, veh/h | 57 | 7 | 183 | 57 | 18 | 194 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 7 | - | 4 | - | - | 3 |
| Peak Hour Factor | 76 | 76 | 76 | 76 | 76 | 76 |
| Heavy Vehicles, \% | 6 | 0 | 6 | 6 | 7 | 6 |
| Mvmt Flow | 75 | 9 | 241 | 75 | 24 | 255 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 582 | 279 | 0 | 0 | 316 | 0 |
| Stage 1 | 279 | - | - | - | - | - |
| Stage 2 | 303 | - | - | - | - | - |
| Critical Hdwy | 7.86 | 6.9 | - | - | 4.17 | - |
| Critical Hdwy Stg 1 | 6.86 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 6.86 | - | - | - | - | - |
| Follow-up Hdwy | 3.554 | 3.3 | - | - | 2.263 | - |
| Pot Cap-1 Maneuver | 374 | 724 | - | - | 1216 | - |
| Stage 1 | 681 | - | - | - | - | - |
| Stage 2 | 658 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 365 | 724 | - | - | 1216 | - |
| Mov Cap-2 Maneuver | 365 | - | - | - | - | - |
| Stage 1 | 681 | - | - | - | - | - |
| Stage 2 | 643 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 16.9 |  | 0 |  | 0.7 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 386 | 1216 | - |
| HCM Lane V/C Ratio |  | - | - | 0.218 | 0.019 | - |
| HCM Control Delay (s) |  | - | - | 16.9 | 8 | 0 |
| HCM Lane LOS |  | - | - | C | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.8 | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | - | ric |  |
| Traffic Vol, veh/h | 74 | 1 | 1 | 60 | 4 | 4 |
| Future Vol, veh/h | 74 | 1 | 1 | 60 | 4 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 1 | - | - | -5 | -7 | - |
| Peak Hour Factor | 80 | 80 | 80 | 80 | 80 | 80 |
| Heavy Vehicles, \% | 5 | 0 | 0 | 4 | 0 | 0 |
| Mvmt Flow | 93 | 1 | 1 | 75 | 5 | 5 |


| Major/Minor | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 94 | 0 | 171 | 94 |
| Stage 1 | - | - | - | - | 94 | - |
| Stage 2 | - | - | - | - | 77 | - |
| Critical Hdwy | - | - | 4.1 | - | 5 | 5.5 |
| Critical Hdwy Stg 1 | - | - | - | - | 4 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 4 | - |
| Follow-up Hdwy | - | - | 2.2 | - | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | - | - | 1513 | - | 880 | 986 |
| Stage 1 | - | - | - | - | 970 | - |
| Stage 2 | - | - | - | - | 980 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1513 | - | 879 | 986 |
| Mov Cap-2 Maneuver | - | - | - | - | 879 | - |
| Stage 1 | - | - | - | - | 970 | - |
| Stage 2 | - | - | - | - | 979 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0.1 |  | 8.9 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) |  | 929 | - | - | 1513 | - |
| HCM Lane V/C Ratio |  | 0.011 | - |  | 0.001 | - |
| HCM Control Delay (s) |  | 8.9 | - | - | 7.4 | 0 |
| HCM Lane LOS |  | A | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.2 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | * |  |  | * | 个 |  |
| Traffic Vol, veh/h | 24 | 54 | 57 | 52 | 22 | 13 |
| Future Vol, veh/h | 24 | 54 | 57 | 52 | 22 | 13 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | \# 0 | - | - | 0 | 0 | - |
| Grade, \% | -2 | - | - | 0 | 0 | - |
| Peak Hour Factor | 80 | 80 | 80 | 80 | 80 | 80 |
| Heavy Vehicles, \% | 10 | 2 | 5 | 5 | 15 | 0 |
| Mvmt Flow | 30 | 68 | 71 | 65 | 28 | 16 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M |  | $\mathbf{F}$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 74 | 27 | 217 | 72 | 14 | 244 |
| Future Vol, veh/h | 74 | 27 | 217 | 72 | 14 | 244 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 7 | - | 4 | - | - | 3 |
| Peak Hour Factor | 99 | 99 | 99 | 99 | 99 | 99 |
| Heavy Vehicles, \% | 0 | 4 | 2 | 2 | 0 | 2 |
| Mvmt Flow | 75 | 27 | 219 | 73 | 14 | 246 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | -1 | ric |  |
| Traffic Vol, veh/h | 88 | 2 | 3 | 100 | 1 | 2 |
| Future Vol, veh/h | 88 | 2 | 3 | 100 | 1 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 1 | - | - | -5 | -7 | - |
| Peak Hour Factor | 88 | 88 | 88 | 88 | 88 | 88 |
| Heavy Vehicles, \% | 2 | 0 | 0 | 1 | 0 | 0 |
| Mvmt Flow | 100 | 2 | 3 | 114 | 1 | 2 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.2 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | -1 | F |  |
| Traffic Vol, veh/h | 16 | 70 | 84 | 38 | 48 | 19 |
| Future Vol, veh/h | 16 | 70 | 84 | 38 | 48 | 19 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | -2 | - | - | 0 | 0 | - |
| Peak Hour Factor | 88 | 88 | 88 | 88 | 88 | 88 |
| Heavy Vehicles, \% | 0 | 3 | 1 | 3 | 4 | 0 |
| Mvmt Flow | 18 | 80 | 95 | 43 | 55 | 22 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | 6 |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 60 | 7 | 183 | 77 | 20 | 194 |
| Future Vol, veh/h | 60 | 7 | 183 | 77 | 20 | 194 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 7 | - | 4 | - | - | 3 |
| Peak Hour Factor | 76 | 76 | 76 | 76 | 76 | 76 |
| Heavy Vehicles, \% | 6 | 0 | 6 | 5 | 6 | 6 |
| Mvmt Flow | 79 | 9 | 241 | 101 | 26 | 255 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 599 | 292 | 0 | 0 | 342 | 0 |
| Stage 1 | 292 | - | - | - | - | - |
| Stage 2 | 307 | - | - | - | - | - |
| Critical Hdwy | 7.86 | 6.9 | - | - | 4.16 | - |
| Critical Hdwy Stg 1 | 6.86 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 6.86 | - | - | - | - | - |
| Follow-up Hdwy | 3.554 | 3.3 | - | - | 2.254 | - |
| Pot Cap-1 Maneuver | 363 | 710 | - | - | 1195 | - |
| Stage 1 | 668 | - | - | - | - | - |
| Stage 2 | 654 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 354 | 710 | - | - | 1195 | - |
| Mov Cap-2 Maneuver | 354 | - | - | - | - | - |
| Stage 1 | 668 | - | - | - | - | - |
| Stage 2 | 638 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 17.6 |  | 0 |  | 0.8 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 374 | 1195 | - |
| HCM Lane V/C Ratio |  | - | - | 0.236 | 0.022 | - |
| HCM Control Delay (s) |  | - | - | 17.6 | 8.1 | 0 |
| HCM Lane LOS |  | - | - | C | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.9 | 0.1 | - |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | $\uparrow$ |  |  | $\$$ |  |  | * |  |  | \$ |  |  |
| Traffic Vol, veh/h | 22 | 74 | 1 | 1 | 60 | 35 | 4 | 0 | 4 | 5 | 0 | 3 |  |
| Future Vol, veh/h | 22 | 74 | 1 | 1 | 60 | 35 | 4 | 0 | 4 | 5 | 0 | 3 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 1 | - | - | -5 | - | - | -7 | - | - | 0 | - |  |
| Peak Hour Factor | 80 | 80 | 80 | 80 | 80 | 80 | 80 | 80 | 80 | 80 | 80 | 80 |  |
| Heavy Vehicles, \% | 0 | 4 | 0 | 0 | 4 | 3 | 0 | 0 | 0 | 20 | 0 | 0 |  |
| Mvmt Flow | 28 | 93 | 1 | 1 | 75 | 44 | 5 | 0 | 5 | 6 | 0 | 4 |  |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.2 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | * |  |  | * | 个 |  |
| Traffic Vol, veh/h | 27 | 56 | 62 | 42 | 22 | 34 |
| Future Vol, veh/h | 27 | 56 | 62 | 42 | 22 | 34 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | \# 0 | - | - | 0 | 0 | - |
| Grade, \% | -2 | - | - | 0 | 0 | - |
| Peak Hour Factor | 80 | 80 | 80 | 80 | 80 | 80 |
| Heavy Vehicles, \% | 12 | 2 | 4 | 5 | 15 | 3 |
| Mvmt Flow | 34 | 70 | 78 | 53 | 28 | 43 |


| Major/Minor M | Minor2 |  | Major1 |  | ajor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 259 | 50 | 71 | 0 | - | 0 |
| Stage 1 | 50 | - | - | - | - | - |
| Stage 2 | 209 | - | - | - | - | - |
| Critical Hdwy | 6.12 | 6.02 | 4.14 | - | - | - |
| Critical Hdwy Stg 1 | 5.12 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.12 | - | - | - | - | - |
| Follow-up Hdwy | 3.608 | 3.318 | 2.236 | - | - | - |
| Pot Cap-1 Maneuver | 729 | 1021 | 1517 | - | - | - |
| Stage 1 | 953 | - | - | - | - | - |
| Stage 2 | 822 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 690 | 1021 | 1517 | - | - | - |
| Mov Cap-2 Maneuver | 690 | - | - | - | - | - |
| Stage 1 | 902 | - | - | - | - | - |
| Stage 2 | 822 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | NB |  | SB |  |
| HCM Control Delay, s | 9.6 |  | 4.5 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT EBLn1 |  | SBT | SBR |
| Capacity (veh/h) |  | 1517 | - | 883 | - | - |
| HCM Lane V/C Ratio |  | 0.051 | - | 0.117 | - | - |
| HCM Control Delay (s) |  | 7.5 | 0 | 9.6 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0.2 | - | 0.4 | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.9 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 90 | 29 | 217 | 74 | 14 | 244 |
| Future Vol, veh/h | 90 | 29 | 217 | 74 | 14 | 244 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 7 | - | 4 | - | - | 3 |
| Peak Hour Factor | 99 | 99 | 99 | 99 | 99 | 99 |
| Heavy Vehicles, \% | 0 | 4 | 2 | 1 | 0 | 2 |
| Mvmt Flow | 91 | 29 | 219 | 75 | 14 | 246 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 531 | 257 | 0 | 0 | 294 | 0 |
| Stage 1 | 257 | - | - | - | - | - |
| Stage 2 | 274 | - | - | - | - | - |
| Critical Hdwy | 7.8 | 6.94 | - | - | 4.1 | - |
| Critical Hdwy Stg 1 | 6.8 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 6.8 | - | - | - | - | - |
| Follow-up Hdwy | 3.5 | 3.336 | - | - | 2.2 | - |
| Pot Cap-1 Maneuver | 417 | 739 | - | - | 1279 | - |
| Stage 1 | 715 | - | - | - | - | - |
| Stage 2 | 698 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 412 | 739 | - | - | 1279 | - |
| Mov Cap-2 Maneuver | 412 | - | - | - | - | - |
| Stage 1 | 715 | - | - | - | - | - |
| Stage 2 | 689 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 15.5 |  | 0 |  | 0.4 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - |  | 462 | 1279 | - |
| HCM Lane V/C Ratio |  | - | - | 0.26 | 0.011 | - |
| HCM Control Delay (s) |  | - | - | 15.5 | 7.8 | 0 |
| HCM Lane LOS |  | - | - | C | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 1 | 0 | - |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | \$ |  |  | \& |  |  | * |  |  | \$ |  |  |
| Traffic Vol, veh/h | 2 | 85 | 2 | 3 | 100 | 4 | 1 | 0 | 2 | 28 | 0 | 18 |  |
| Future Vol, veh/h | 2 | 85 | 2 | 3 | 100 | 4 | 1 | 0 | 2 | 28 | 0 | 18 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 1 | - | - | -5 | - | - | -7 | - | - | 0 | - |  |
| Peak Hour Factor | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 |  |
| Heavy Vehicles, \% | 0 | 2 | 0 | 0 | 1 | 20 | 0 | 0 | 0 | 4 | 0 | 0 |  |
| Mvmt Flow | 2 | 97 | 2 | 3 | 114 | 5 | 1 | 0 | 2 | 32 | 0 | 20 |  |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.7 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | * ${ }^{\text {F }}$ |  |  | $\uparrow$ | 个 |  |
| Traffic Vol, veh/h | 33 | 81 | 85 | 38 | 49 | 22 |
| Future Vol, veh/h | 33 | 81 | 85 | 38 | 49 | 22 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | -2 | - | - | 0 | 0 | - |
| Peak Hour Factor | 88 | 88 | 88 | 88 | 88 | 88 |
| Heavy Vehicles, \% | 3 | 3 | 1 | 3 | 4 | 5 |
| Mvmt Flow | 38 | 92 | 97 | 43 | 56 | 25 |


| Major/Minor M | Minor2 |  | Major1 |  | ajor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 306 | 69 | 81 | 0 | - | 0 |
| Stage 1 | 69 | - | - | - | - | - |
| Stage 2 | 237 | - | - | - | - | - |
| Critical Hdwy | 6.03 | 6.03 | 4.11 | - | - | - |
| Critical Hdwy Stg 1 | 5.03 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.03 | - | - | - | - | - |
| Follow-up Hdwy | 3.527 | 3.327 | 2.209 | - | - | - |
| Pot Cap-1 Maneuver | 708 | 995 | 1523 | - | - | - |
| Stage 1 | 959 | - | - | - | - | - |
| Stage 2 | 821 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 662 | 995 | 1523 | - | - | - |
| Mov Cap-2 Maneuver | 662 | - | - | - | - | - |
| Stage 1 | 897 | - | - | - | - | - |
| Stage 2 | 821 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | NB |  | SB |  |
| HCM Control Delay, s | 9.9 |  | 5.2 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT EBLn1 |  | SBT | SBR |
| Capacity (veh/h) |  | 1523 | - | 869 | - | - |
| HCM Lane V/C Ratio |  | 0.063 | - | 0.149 | - | - |
| HCM Control Delay (s) |  | 7.5 | 0 | 9.9 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0.2 | - | 0.5 | - | - |


[^0]:    ${ }^{1}$ Estimates were determined using LUC 210 - "Single-Family Detached Housing"
    ${ }^{2}$ AM Calibration Factor $=1.1 \mid$ PM Calibration Factor $=1.0$

[^1]:    ${ }^{3}$ The Fitted Curve results were utilized in accordance with the ITE guidance to use those results when there are more than 20 studies for the land use.

[^2]:    42
    Out: 150 In: 180
    [E] Lake Station Rd

